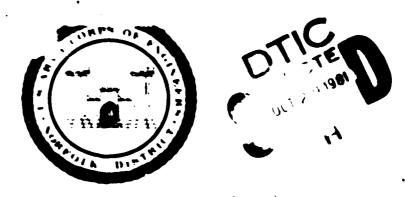
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PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM.



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20. Abstract

Pursuant to Public Law 92-367, Phase I Inspection Reports are prepared under guidance contained in the recommended guidelines for safety inspection of dams, published by the Office of Chief of Engineers, Washington, D. C. 20314. The purpose of a Phase I Inspection is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general conditions of the dam is based upon available data and visual inspection. Detailed investigation and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

Based upon the field conditions at the time of the field inspection and all available engineering data, the Phase I report addresses the hydraulic, hydrologic, geologic, geotechnic, and structural aspects of the dam. The engineering techniques employed give a reasonably accurate assessment of the conditions of the dam. It should be realized that certain engineering aspects cannot be fully analyzed during a Phase I inspection. Assessment and remedial measures in the report include the requirements of additional indepth study when necessary.

Phase I reports include project information of the dam appurtenances, all existing engineering data, operational procedures, hydraulic/hydrologic data of the watershed, dam stability, visual inspection report and an assessment including required remedial measures.

PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of the Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation and analyses involving topographic mapping, subsurface investigations testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established guidelines, the spillway design flood is based on the estimated "Probable Maximum Flood" for the region (flood discharges that may be expected from the most severe combination of critical meteorologic and hydrologic conditions that are reasonably possible), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the design flood should not be interpreted as necessarily posing a highly inadequate condition. The design flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition, and the downstream damage potential.

PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM

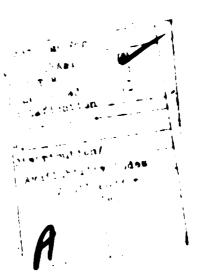
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NAME OF DAM: LAKE HOLLY DAM

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PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM

Name of Dam: Lake Holly Dam State: Commonwealth of Virginia

County: Caroline

USGS 7.5 Minute Quadrangle: Penola, Virginia

Stream: Unnamed Tributary to the Maracossic Creek

Date of Inspection: 15 May 1981

BRIEF ASSESSMENT OF DAM

Lake Holly Dam is an earthfill embankment approximately 20.7 feet high and 365 feet long. The principal spillway is a corrugated metal pipe riser located near the center of the embankment. An emergency spillway is located on the right abutment. The dam, located about 14.5 miles west of Sparta, Virginia, is used for recreation. The dam is owned by R.L. Stansberry, P.O. Box 245, Annandale, Virginia 22003. Lake Holly Dam is a "small" size - "significant" hazard structure as defined by the Recommended Guidelines for Safety Inspection of Dams. The dam and appurtenant structures were in good overall condition at the time of inspection. Maintenance of the dam is considered to be adequate. A stability check of the dam is not required.

Using the Corps of Engineers' screening criteria for initial review of spillway adequacy, the 100-year flood was selected as the spillway design flood (SDF). The spillway is capable of passing up to 89 percent of the SDF or 21 percent of the Probable Maximum Flood (PMF) without overtopping the dam. Overtopping velocities are not considered to be seriously detrimental to the dam. The spillway is adjudged as inadequate, but not seriously inadequate.

Regular inspections should be made of the dam and appurtenant structures. A thorough check list should be compiled for use by the owner's representative as a guide for the inspections. Maintenance items should be completed annually.

The erosion of the upstream face of the dam should be monitored.

A formal warning system and emergency action plan should be developed and implemented as soon as possible.

Measured from the streambed at the downstream toe to the embankment crest.

²Facing downstream.

The following repair items should be accomplished as part of the general maintenance of the dam

- Remove trees and debris from the outlet channel. 1)
- 2) Backfill, compact, and protect from further erosion the scour area below the spillway outlet.
- 3) Remove the trees and brush from the embankment and downstream toe area by cutting them off at ground Trees with a trunk diameter greater than 3 inches should also have their root systems removed. The resulting holes should be backfilled, compacted, regraded, and seeded
- 4) Place erosion protection in the emergency spillway.
- 5) Install a staff gage to monitor reservoir levels above normal pool.

MICHAEL BAKER JR., INC. SUBMITTED

riginal signed by: Strick Anteron, Jr.

Carl S. Anderson, Jr., P.E. Acting Chief, Design Branch

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Original wid JAMES A. VALUH

Jack G. Starr, P.E Chief, Engineering

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APPROVED

Ronald E. Hudson

Colonel, Corps of Engineers

District Engineer

Date.

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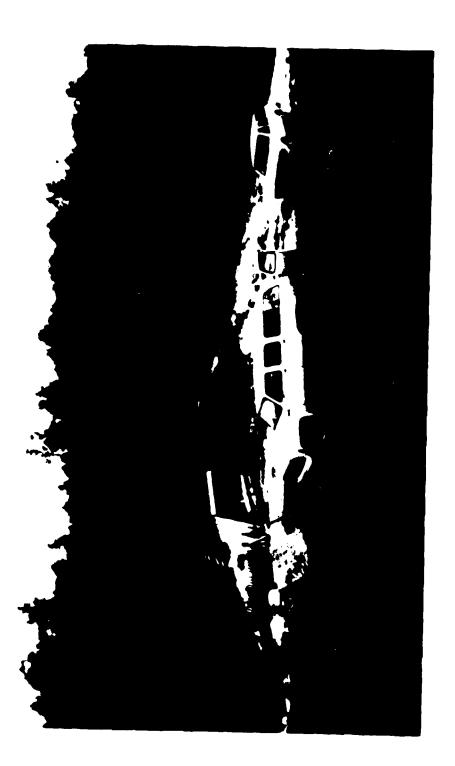
MICHAEL BAKER III NO 3176

Michael Baker, III, P.E.

Chairman of the Board and Chief Executive Officer

NAME OF DAM: LAKE HOLLY DAM

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OVERALL VIEW OF DAM

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PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM NAME OF DAM: LAKE HOLLY DAM ID# VA 03334

SECTION 1 - PROJECT INFORMATION

1.1 General

- 1.1.1 Authority: Public Law 92-367, 8 August 1972, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a national program of safety inspections of dams throughout the United States. The Norfolk District has been assigned the responsibility of supervising the inspection of dams in the Commonwealth of Virginia.
- 1.1.2 Purpose of Inspection: The purpose is to conduct a Phase I inspection according to the Recommended Guidelines for Safety Inspection of Dams (Reference 12, Appendix IV). The main responsibility is to expeditiously identify those dams which may be a potential hazard to human life or property.

1.2 Description of Project

1.2.1 Description of Dam and Appurtenances: Lake Holly Dam is an earthfill embankment approximately 20.7 feet high and 365 feet long. The crest of the dam is about 12.5 feet wide, and the minimum crest elevation is 1006 feet Temporary Bench Mark (T.B.M.)2. The slope of the upstream face of the embankment varies from 2.2H:1V (Horizontal to Vertical) to 3.4H:1V. The slope of the downstream face of the embankment varies from 3.0H:1V to 3.5H:1V. There is no information available on any possible zoning of the embankment or any foundation drainage system. A wooden breakwater was installed approximately two years ago to protect the upstream slope from erosion.

Measured from the streambed at the downstream toe to the embankment crest.

²All elevations are referenced to a Temporary Bench Mark located on top of the corrugated metal pipe intake riser. The assumed elevation is 1000.0 feet.

The principal spillway is located approximately 205 feet from the left abutment and 35 feet upstream from the crest of the dam. spillway is comprised of two vertical sections of corrugated metal pipe (CMP). The first section is a 42-inch diameter CMP riser with a crest elevation of 1000.0 feet T.B.M. Attached to this section is a 1-1/2 foot section of 54-inch diameter CMP that extends the riser to an elevation of 1000.9 feet T.B.M. Water can enter the principal spillway at elevation 1000.0 feet T.B.M. by flowing between the two pipes, and at elevation 1000.9 feet T.B.M. by flowing over the 54-inch Water entering the principal spillway is carried through the embankment by means of a 30-inch diameter CMP that extends through the embankment to the downstream channel. The invert elevation at the outlet of this pipe is 986.9 feet T.B.M.

The water surface of the lake can be lowered 5 feet by a gate 21 inches high and 42 inches wide on the spillway riser. The gate is operated by a chain attached to the top of the gate.

The emergency spillway is located on the right abutment and is a trapezoidal shaped vegetated channel. The spillway is approximately 60 feet wide and has a crest elevation of 1003.7 feet T.B.M. A dirt roadway providing access to the dam and reservoir passes through the emergency spillway and ends at the upstream edge of the reservoir.

Lake Holly Dam has a drainage area of 1.16 square miles located south and west of the dam. Thelma Pitts Dam is located 2900 feet downstream. Thelma Pitts reservoir is 800 feet downstream of Lake Holly Dam.

1.2.2 Location: Lake Holly Dam is located in Caroline County, Virginia on an unnamed tributary of the Maracossic Creek, approximately 1.4 miles west of Sparta, Virginia. A Location Plan is included with this report in Appendix I.

- 1.2.3 Size Classification: The height of the dam is 20.7 feet, and the reservoir storage capacity at the crest of the dam (elevation 1006 feet T.B.M.) is 246 acre-feet. Therefore, the dam is in the "small" size category as defined by the Recommended Guidelines for Safety Inspections of Dams.
- 1.2.4 Hazard Classification: Thelma Pitts reservoir is located 800 feet downstream, and Thelma Pitts Dam is 2900 feet downstream from Lake Holly Dam. Virginia Route 654 crosses the downstream channel over a 4-foot diameter concrete culvert 3100 feet downstream. The Sparta Volunteer Fire Company house is located 4000 feet downstream in the floodplain. Loss of life in the event of dam failure is not considered highly probable. Economic losses due to damage of Thelma Pitts Dam, Virginia Route 654, the access road to the fire house, and the fire house are likely in the event of a dam failure. Lake Holly Dam is, therefore, considerd to be in the "significant" hazard category as defined by the Renommended Guidelines for Catety Inspections of Tams The hazard classification used to categorize dama is a function of location only and is not related to other lity or prorability of factore
- 1.2 5 wherehigh The dam and releavour are when by R.D. Standaerry E. E.X. (4) Administrate Virginia 22.00
- 1.2 The where the rations of the process of and a control of the control of any section of the control of any section of the control of the c

once a year for maintenance of the upstream face of the dam. No formal operating procedures are followed for this structure.

1.3 Pertinent Data

- 1.3.1 <u>Drainage Area:</u> The total drainage area contributing to Lake Holly Dam is 1.16 square miles.
- 1.3.2 Discharge at Dam Site: The maximum discharge is unknown.

Pool level at minimum top of dam:

Principal Spillway 70 c.f.s. Emergency Spillway 816 c.f.s.

1.3.3 Dam and Reservoir Data: Pertinent data on the dam and reservoir are provided in the following table:

TABLE 1.1 DAM AND RESERVOIR DATA

	Reservoir Capacity				
1.000					
	Elevation (feet T E.M.)	Area (acres)		Watershed (inches)	-
T 1 43#	1006.0	27.3	246.0	4.0	3320]
Emeropera (Frances)	1003	20.9	188.0	3.0	3160.0
otil oja. Ojileay otest Oteanielst	1000	14.4	123.0	2.0	2880.C
* *	985.3	-			-

SECTION 2 - ENGINEERING DATA

- 2.1 Design: Design plans, specifications, and boring logs were not available for use in preparing this report. No stability analyses or hydrologic and hydraulic data were available for review.
- 2.2 Construction: Construction records, as-built plans, and inspection logs were not available for review.
- 2.3 Evaluation: There is insufficient information to evaluate foundation conditions and embankment stability. No design or construction records were available for use in assessing the condition of the dam. All evaluations and assessments in this report were based upon field observations, conversations with the owner, and office analyses.

SECTION B - VISUAL INSPECTION

3 l Finalings

- 3 1 1 Jene:al The field inspection was conducted on 15 May 1981. At the time of the inspection the pool elevation was 1000+ feet T B M , the tailwater elevation was 986 T feet T B.M., and the weather was lear with a temperature of 70 degrees Fahrenheit. The ground surface on the empankment and the abutments were dry. The sam and appurtenant stru tures were found to be in good overall condition at the time of the inspection. Deficiencies found during the inspection will require remedial treatment The following are brief summaries of these deficiencies. A Field Sketch of conditions found during the inspection is presented as Plate 1 in appendix I. The complete visual inspection check list is provided in Appendix III. No record was found of any previous inspections
- Dam. The embankment was found to be in generally good condition with no surface cracks of slides observed. The embankent is vegetated with grass. There are trees and brush growing at scattered locations on the downstream face of the dam. This growth is particularly dense along the downstream toe. The upstream face of the embankment was slightly eroued at normal pool level. A wooden freakwater was installed to help minimize the erosion.
- Appurtenant Structures: The principal spillway (as described in Section 1.2.1) was found to be in good condition. The area downstream from the outlet pipe has been scoured out by discharge through the outlet pipe.

The energency outlet gate consists of the 21inon by 41-inon gate that is opened by means of a chain. The gate was submerged at the time of the inspection and could not be observed.

NAME FOAY LAFE HOLLY DAM

The approach area to the emergency spil.way is broad and uncostructed. The approach area consists of matura, mound and reservoir bottom with little venetation. The discharge hannel is not well defined downstream of the crest. A dirt coadway providing access to the dam and reservoir passes through the emergency spillway and ends at the upstream educ of the reservoir.

)

- Reservoir Area. The area surrounding the reservoir is mildly sloping. The slopes are wooded and in good condition with no evidence of eresion. Soundings at the time of the inspection show the water to be 12 feet deep at a distance of 100 feet from shore near the center of the embankment. No significant accumulations of debris were observed in the reservoir area.
- Downstream Channel. The downstream channel has very mild slopes. The channel is overgrown with trees and brush and contains some debris. The stream enters Thelma Pitts reservoir (ID No. VA 03343) about 800 feet downstream. Approximately 200 feet below Themla Pitts reservoir, the stream passes under Virginia Route 654 through a 4-foot diameter culvert An additional 600 feet downstream the channel passes through a 4-foot diameter culvert under the access road at the Sparta Volunteer Fire Company.
- 3.1.b Instrumentation. There was no instrumentation present at the dam.
- 3 2 Evaluation: In general, the dam and appurtenant structures were in good condition. The outlet channel should be cleared of trees and debris. The scoured area near the outlet pipe should be repaired and protected from further erosion. Erosion protection should be provided for the emergency spillway. The erosion of the upstream face is not considered to be a serious problem at this time but should be monitored. The trees and brush should be removed from the empankment and area along the downstream toe.

A staff gage should be installed to monitor reservoir levels above normal pool

SECTION 4 - OFERATIONAL PROCEDURES

- 4.1 Procedures: Operation of the dam is an automatic function controlled by the principal spillway. Water entering the reservoir flows into the principal spillway at elevation 1000.0 feet T.B.M. The emergency spillway is activated when the reservior level rises above 1003.7 feet T.B.M. The reservoir can be lowered approximately 5 feet to elevation 995.0 feet T.B.M. by the operation of the gate on the principal spillway.
- 4.2 Maintenance of Dam: Maintenance of the dam is the responsbillity of the owner. An inspection or maintenance schedule has not been instituted.
- 4.3 Maintenance of Operating Facilities: The operating facility is the gate which lowers the reservoir 5 feet. The owner is responsible for maintenance of this facility. A formal inspection schedule has not been instituted.
- 4.4 Warning System: At the time of inspection, there was no warning system or emergency plan in operation.
- 4.5 Evaluation: Maintenance of the dam in the past has been adequate. Regular inspections of the dam and appurtenant structures should be made and documented. A thorough check list should be compiled for use by the owner's representative as a guide for the inspections. Maintenance items should be corrected annually. A warning system and emergency action plan should be developed and implemented as soon as possible. The plan should include:
 - a. How to operate the dam during an emergency.
 - b. Who to notify, including public officials, in case evacuation from the downstream area becomes necessary.

The local Emergency Services Coordinator of the State Office of Energy and Emergency Services can assist in the preparation of an emergency warning plan.

SECTION 5 - HYDRAULIC/HYDROLOGIC DATA

- 5.1 Design: No design data were available for use in preparing this report.
- 5.2 Hydrologic Information: No rainfall, stream gage or reservoir stage records are maintained for this dam.
- 5.3 Flood Experience: No records were available.
- 5.4 Flood Fotential: The Probable Maximum Flood (PMF), 1 2 Probable Maximum Flood (1 2 FMF), and 100-year flood were developed and routed through the reservoir by use of the HEC-1 DB computer program (Reference 9, Appendix IV) and appropriate unit hydrograph, precipitation and storage-outflow data. Clark's T_c and R coefficients for the local drainage areas were estimated from basin characteristics. The rainfall applied to the unit hydrograph was taken from publications by the U.S. Weather Bureau and the National Oceanic and Atmospheric Administration (References 16 and 17, Appendix IV). Rainfall losses for the PMF were estimated at an initial loss of 1.0 inch and a constant loss rate of 0.05 inch per hour thereafter. Rainfall losses for the 100-year flood were estimated at an initial loss of 1.5 inches and a constant loss rate of 0.15 inch per hour thereafter
- 5.5 Reservoir Regulation: Pertinent dam and reservoir data are provided in Table 1.1. Paragraph 1.3.3.

Regulation of flow from the reservoir is automatic. Normal flows are maintained by the crest of the principal spillway at elevation 1000.0 feet T.B.M. Some manual regulation of flow is possible by opening the gate on the riser pipe. By opening this gate, the water surface can be lowered to 995.0 feet T.B.M. Water may also be discharged through the emergency spillway when the reservoir rises above elevation 1003.7 feet T.B.M.

Outlet discharge capacity was computed by hand. Reservoir area was estimated from the Fenola, Virginia, 7.5 minute USGS quadrangles, and storage capacity curves above normal pools were computed by the HEC-1 DB program. All flood routings began with the reservoir at normal pool (1808 & reet T.B.M.) Flow through the principal spillway was included in the routings

5.6 Overtopping Potential: The probable rise of the reservoir and other pertinent information on reservoir performance are shown in the following table:

TABLE 5.1 RESERVOIR PERFORMANCE

	······································	ydrograph	phs	
Item	Normal ¹	100-Year flood	1/2 PMF	PMF ²
Peak flow, c.f.s.				
Inflow	1.0	2235.0	5829.0	11,657.0
Outflow	1.0	1398.0	5416.0	11,066.0
Peak elev., ft. T.B.M.	1000.0	1006.8	1008.9	1,010.4
Non-overflow section				•
(elev. 1006.0 ft. T.B.M.)				
Depth of flow, ft.	_	0.8	2.9	4.4
Average velocity, f.p.s.	•	4.1	7.9	9.7
Total duration of over-				
topping, hrs.	-	1.2	3.8	5.5
Tailwater elev., ft. T.B.M	. 985.3	-	-	-

'Conditions at time of inspection.

¹Velocity estimatés were based on critical depth at control section

- Reservoir Emptying Potential: The reservoir can be lowered 5 feet by opening the gate on the primary spillway. Neglecting inflow, the reservoir can be drawn down 5 feet from normal pool in approximately 60 hours. This is equivalent to an approximate drawdown rate of 2.0 feet per day, based on the hydraulic height measured from normal pool to the spillway gate divided by the time to lower the reservoir 5 feet.
- 5.8 Evaluation: Lake Holly Dam is a "small" size "significant" hazard dam requiring evaluation for a spillway design flood (SDF) in the range of the 100-year flood to the 1-2 PFM. Due to the risk involved, the 100-year flood was selected as the SDF. The 100-year flood was routed through the reservoir and found to overtop the dam by a maximum depth of 0.8 feet with an average

The PMF is an estimate of flood discharges that may be expected from the most severe combination of critical meterologic and hydrologic conditions that are reasonably possible in a region.

critical velocity of 4.1 feet per second (f.p.s.). Total duration of dam overtopping would be 1.2 hours. The spillways are capable of passing up to 21 percent of the PMF or 89 percent of the SDF without overtopping the dam.

Conclusions pertain to present day conditions, and the effect of future development on the hydrology has not been considered.

SECTION 6 - DAM STABILITY

6.1 Foundation and Abutments: No information is available on foundation conditions other than observations made at the time of the inspection. The dam is located in the Atlantic Coastal Plain geologic region, and the predominant deposit in the area of the dam is Tertiary Age Calvert Formation consisting of interbedded sand and clay. The Geologic Map of Virginia also shows Quaternary-age upland gravel and sand existing at the surface of the area.

6.2 Embankment

- 6.2.1 Materials: There was no information describing the nature of the materials or any zoning within the embankment. However, Mr. R.L. Stansberry, the present owner, reports a clay core does exist within the embankment. The outer embankment was found to be fine to very fine grained sand with some traces of silt.
- Stability: Design plans and previous stability analysis results were unavailable for this inspection. The dam is 20.7 feet high with a crest width of 12.5 feet. The upstream slope was measured to vary from 2.2H:1V to 3.4H:1V. The downstream slope was measured to be 3.1H:1V. The outlet facilities provide the capability to drain the reservoir 5 feet in the event of an emergency or for maintenance purposes. As presented in Section 5.7, the maximum possible drawdown rate is 2 feet per day. Therefore, if not regulated, a rapid drawdown situation is possible.

According to the guidelines presented in Design of Small Dams by the U.S. Department of the Interior, Bureau of Reclamation, for small dams of the described material with stable foundations subject to rapid drawdown, the recommended side slopes are 3.5H:1V for the upstream face and 2.5H:1V for the downstream face. The recommended crest width is 14.0 feet. The downstream slope is within these guidelines while the upstream slope is steeper and the crest width is narrower than recommended.

Other than the minor erosion occurring on the upstream face of the embankment along normal pool level, no signs of instability were observed.

- 6.2.3 Seismic Stability: The dam is located in Seismic Zone I, which presents no hazard from earthquakes according to the Recommended Guidelines for Safety Inspection of Dams by the Department of the Army, Office of the Chief of Engineers. This determination is contingent on the requirements that static stability conditions are satisfactory and conventional safety margins exist.
- 6.3 Evaulation: The results of previous stability analyses were unavailable for review for this report. The downstream slope is within the Bureau of Reclamation guidelines, while the upstream slope is steeper and the crest width is narrower than the recommended values. However, no significant signs of instability were noted.

As described in Section 5 of this report, the dam would be overtopped by the SDF. Despite the inability of the spillway to pass the SDF, the depth, and duration, and rate of overtopping are not considered to be seriously detrimental to the embankment. Overtopping flows are shallow and last only 1.2 hours, and the velocity is less than 6.0 f.p.s., the effective eroding velocity for a vegetated earth embankment. A stability check is not required.

SECTION 7 - ASSESSMENT REMEDIAL MEASURES

7.1 Dam Assessment: There is insufficient information to evaluate foundation conditions and embankment stability. There was no engineering data available for use in preparing this report. Deficiencies discovered during the field inspection and the office analyses require remedial treatment. The dam and appurtenant structures are generally in good condition. Maintenance is considered adequate. A stability check of the dam is not required.

Using the Corps of Engineers' screening criteria for initial review of spillway adequacy, the 100-year flood was selected as the SDF for the "small" size - "significant" hazard classification of Lake Holly Dam. The spillway is capable of passing up to 89 percent of the SDF or 21 percent of the FMF without overtopping the non-overflow section of the dam. The SDF was found to overtop the dam by a maximum depth of 0.8 feet with an average critical velocity of 4.1 feet per second (f.p.s.). The overtopping flows are not considered to be seriously detrimental to the embankment. Therefore, the spillway is adjudged as inadequate, but not seriously inadequate.

There is no flood warning system or emergency action plan currently in operation.

7.2 Recommended Remedial Measures: Regular inspections should be made of the dam and appurtenant structures. A thorough check should be compiled for use by the owner's representative as a guide for the inspections. Maintenance items should be completed annually.

The erosion of the upstream face of the embankment should be monitored.

A formal warning system and emergency action plan should be developed and implemented as soon as possible.

The following repair items should be accomplished as part of the general maintenance of the dam.

- Remove trees and debris from the outlet channel.
- 2) Backfill, compact, and riprap the scour area below the spillway outlet.

- 3) Remove the trees and brush from the embankment and downstream toe area by cutting them off at ground level. Trees with a trunk diameter greater than 3 inches should also have their root systems removed and the resultant holes backfilled, compacted, regraded, and seeded.
- 4) Place erosion protection in the emergency spillway.
- 5) Install a staff gage to monitor reservoir levels above normal pool.

APPENDIX I PLATES

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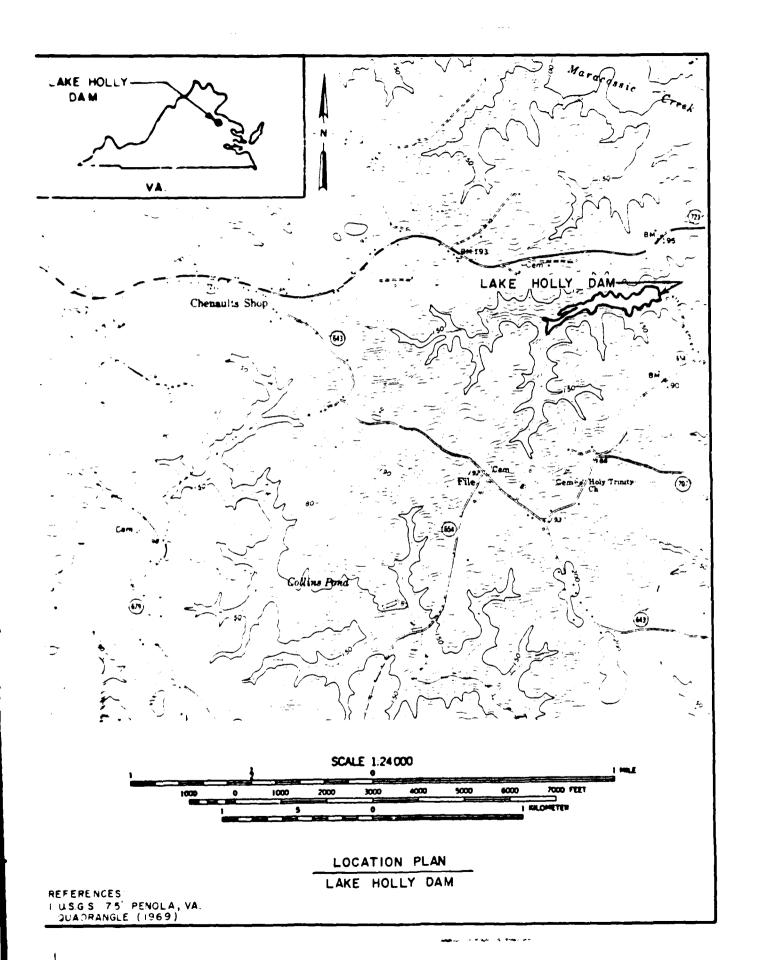
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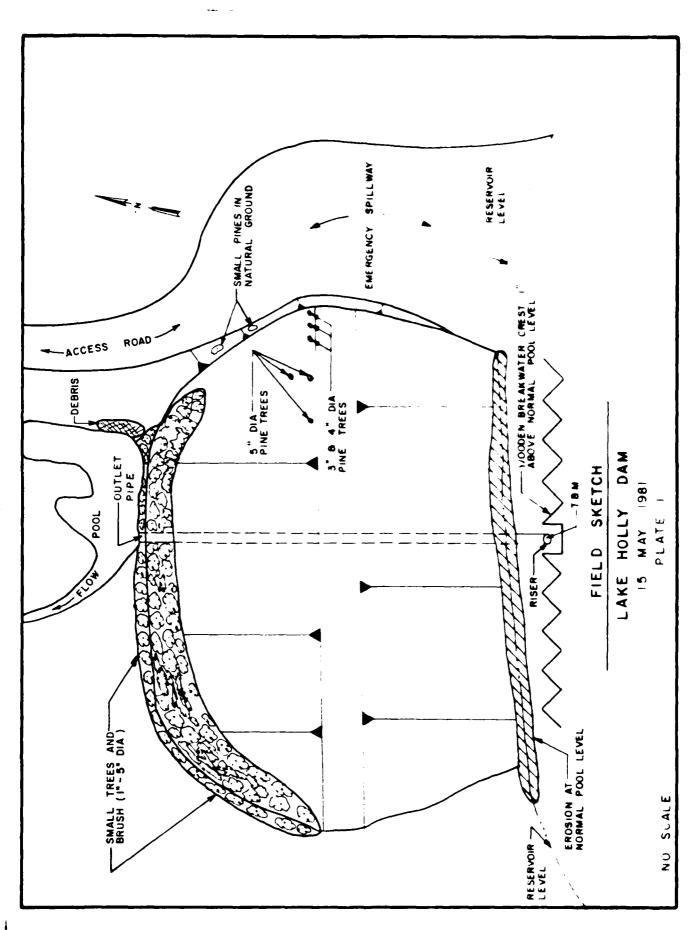
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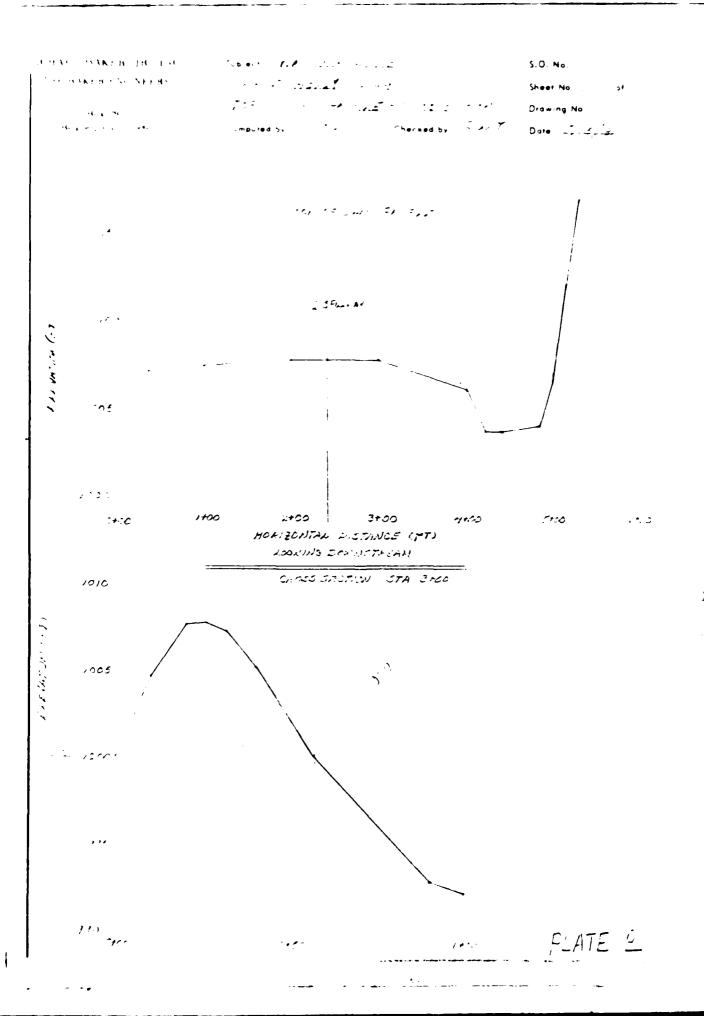
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MICHAEL BAKER, JR., INC. Not been and admit with THE BAKER ENGINEERS March Sant Carlot Here 200 Heaver, Harrison

APPENDIX II

PHOTOGRAPHS

CONTENTS

Photo 1: Upstream Face of Embankment

Photo 2: Downstream Face of Embankment

Photo 3; Riser Intake and Wooden Breakwater

Photo 4: Outlet Pipe

Photo 5: Erosion of Upstream Face

Photo 6: Emergency Spillway Channel (Looking Upstream)

Note: Photographs were taken on 15 Many 1981.

LAKE HOLLY DAM



PHOTO 1. Upstream Face of Embankment



PHOTO 2. Downstream Face of Embankment

LAKE HOLLY DAM



PHOTO 3. Riser Intake and Wooden Breakwater



PHOTO 4. Outlet Pipe

LAKE HOLLY DAM

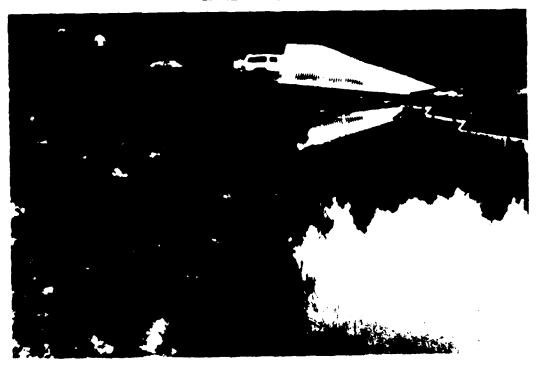


PHOTO 5. Erosion of Upstream Face



PHOTO 6. Emergency Spillway Channel (Looking Upstream)

APPENDIX III

VISUAL INSPECTION CHECK LIST

Check List Visual Inspection Phase 1

Name of Dam Lake Holly Dam	y Dam County Caroline	State	State Virginia Coordinates Lat. 3759.1 Long. 7715.3
Date of Inspection	15 May 1981	Weather	Sunny Temperature 70°F.
Pool Elevation at Time	Pool Elevation at Time of Inspection 1000.+ T.B.M.	. в. ж.	Tailwater at Time of Inspection 986.7 T.B.1
Inspection Personnel:	Michael Baker, Jr., Inc.:	":	Owner's Representatives:
			Mr. R. L. Stansberry
	Steve M. Lockington Dave W. Miller		Virginia State Water Control

Hugh Gildea Leon Musselwhite

Virginia State Water Control Board

Wayne D. Lasch

Recorder

þ

EMBANKMENT

Name of Dam LAKE HULLY DAM

OBSERVATIONS VISUAL EXAMINATION OF

REMARKS OR RECOMMENDATIONS

SURFACE CRACKS

The ground surface of the embankment and the abutments were dry. No surface cracks were observed.

UNUSUAL MOVEMENT OR CRACKING AT OR BEYOND THE TOE

None were observed.

SLOUGHING OR EROSION OF EMBANKMENT AND ABUTMENT SLOPES

A limited amount of erosion was observed on the upstream face of the embankment at normal pool level. The owner installed a wooden breakwater in the reservoir immediately upstream from the dam to minimize the progression of this erosion. The breakwater was installed two years ago.

The erosion is not considered to be a major problem at this time. However, it should be monitored and, if it becomes more serious, erosion protection should be placed on the upstream face of the dam.

EMBANKMENT

Name of Dam LAKE HOLLY DAM

VISUAL EXAMINATION OF VERTICAL AND HORIZONTAL	No problems were observed.	REMARKS OR RECOMMENDATIONS
	the property and the contraction of the contraction	
Control Control	•	

I RIPRAP FAILURES

This is not applicable.

Name of Dam LAKE HOLLY DAM

VISUAL EXAMINATION OF	OBSERVATIONS REMARKS OF DECOMMENDATIONS
JUNCTION OF EMBANKMENT AND ABUTMENT, SPILLWAY AND DAM	There are trees and brush growing The trees and brush should at scattered locations on the be removed. embankment. This growth is particularly dense above the downstream toe. No other problems observed.
ANY NOTICEABLE SEEPAGE	No seepage was observed.
III - 4	
STAFF GAGE AND RECORDER	There were none. A staff gauge should be installed to monitor reservoir levels above normal pool.

There were none.

DRA INS

Name of Dam: LAKE HOLLY DAM

OBSERVATIONS REMARKS OF RECOMMENDATIONS	This is not applicable.	The intake structure consists of a 42 in. diameter CMF riser unit. This structure is an good condition. There is a 21 in. by 42 in slide gate at the base of the intake structure. This gate is operated by using a heavy chain connected to the gate to pull the gate open. The gate was submerged during the inspection and could not be observed.	There is an area downstream from the from the outlet conduit out by discharge through the outlet should be billed. The area conduit. This area is about 5 ft. should be protected to prewide by 5 ft. deep by 5 ft. long.	The outlet channel is a natural channel which has an extremely mild improved to prevent water from slope. The channel is also overable of the with some debris. The combination of channel slope, dense vegetation, and debris cause water to pond alone the downstream toe of the dam.
VISUAL EXAMINATION OF	CHACKING AND SPALLING OF CONCRETE SURFACES IN OUTLET CONDUIT	INTAKE STRUCTURE	OUTLET STRUCTURE	OUTLET CHANNEL

UNGATED SPILLWAY - EMERGENCY SPILLWAY

Name of Dam: LAKE HOLLY DAM

REMARKS OR RECOMMENDATIONS OBSERVATIONS VISUAL EXAMINATION OF

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EMERCH WOLLS A.E.

COMMENS and a this is not applicable.

Spillway consists of the midly shound natural ground and reserved bettom.

The transfers providing access to the transfer passes through the transferst spillway and ends at the transfer channel is not well attract downstream from the crest in the trest.

ough installed to protect the at emergency spillway from being ir. eroded when it is activated.

Erosion protection should be

A CALL THE THOUGH

INSTRUMENTATION

Name of Dame Cart Holly DAM

VISUAL EXAMINATION	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
M.MUMENTATION/SURVEYS	None were observed.	
OBSERVATION WELLS	There were none.	
WEIRS	There were none.	
PIEZOMETERS	There were none.	
LTHER	There were none.	

There were none.

RESERVOIR

Name of Dam: LAKE HOLLY DAM

VISUAL EXAMINATION OF	OBSERVATIONS RECLEMENTATIONS
810PES	The reservoir slopes are mild. No signs of instability were observed during the inspection. The watershed surrounding the reservoir is primarily forested with some agricultural
	activities.

SEDIMENTATION

Sedimentation is not reported to be a significant problem. Soundings taken during the inspection indicate that the reservoir is approximately 1. It at a distance of 100 ft. from shore near the center of the embankment.

DOWNSTREAM CHANNEL

Name of Dam: LAKE HOLLY DAM

1

VISUAL EXAMINATION OF

OBSERVATIONS

(OBSTRUCTIONS, DEBRIS, ETC.) CONDITION

The downstream channel is mildly sloping and overgrown with vegetation.

REMARKS OR RECOMMENDATIONS

SLOPES

The channel has very mild slopes.

APPROXIMATE NO. OF HOMES AND POPULATION

and contains some debris. The downstream The channel is overgrown with trees and VA 03343) 800 ft. downstream. Approximately 200 ft. below Thelma Pitts reserenters Thelma Pitts reservoir (1.), No. Route 654 through a 4 ft. diameter culvert. An additional 600 ft. downstream, the channel passes through a 4 ft. diavoir, the stream passes under Virginia meter culvert under the access road a the Sparta Volunteer Fire Company. APPENDIX IV
GENERAL REFERENCES

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- Bureau of Reclamation, U.S. Department of the Interior, Design of Small Dams, A Water Resources Technical Publication, Revised Reprint, 1977.
- 2. Chow, Ven Te, Handbook of Applied Hydrology, McGraw Hill Book Company, New York, 1964.
- 3. Chow, Ven Te, Open Channel Hydraulics, McGraw Hill Book Company, New York, First Edition, 1959.
- 4 Commonwealth of Virginia, "Geologic Map of Virginia," Department of Conservation and Economic Development, and Division of Mineral Resources, 1963.
- 5. HR 33, "Seasonal Variations of Probable Maximum Precipitation, East of the 105th Meridian for Areas 10 to 1000 Square Miles and Durations of 6 to 48 Hours," (1956).
- King, Horace Williams and Brater, Ernest F., Handbook of Hydraulics, Fifth Edition, McGraw - Hill Book Company, New York, 1963.
- Soil Conservation Service, "National Engineering Handbook -Section 4, Hydrology," U.S. Department of Agriculture, 1964.
- 8. Soil Conservation Service, "National Engineering Handbook Section 5, Hydraulics," U.S. Department of Agriculture.
- 9. U.S. Army, Hydrologic Engineering Center, "Flood Hydrograph Package (HEC-1), Dam Safety Investigations, Users Manual," Corps of Engineers, Davis, California, September 1978.
- 10. U.S. Army, Hydrologic Engineering Center, "HEC-2 Water Surface Profiles, Users Manual," Corps of Engineers, Davis, California, October 1973.
- 11. U.S. Army, "Inventory of United States Dams," Corps of Engineers, 9 September 1978.
- 12. U.S. Army, Office of the Chief of Engineers, "Appendix D, Recommended Guidelines for Safety Inspection of Dams,"

 National Program of Inspection of Dams, Volume 1, Corps of Engineers, Washington, D.C., May 1975.

NAME OF DAM: LAKE HOLLY DAM

AL STATE OF THE ST

- 13. U.S. Army, Office of the Chief of Engineers, Engineering Circular EC-1110-2-163 (Draft Engineering Manual), "Spillway and Freeboard Requirements for Dams, Appendix C, Hydrometeorological Criteria and Hyetograph Estimates," (August 1975).
- 14. U.S. Army, Office of the Chief of Engineers, Engineering Circular EC-1110-2-188, "Engineering and Design, National Program of Inspection of Non-Federal Dams," Corps of Engineers, Washington, D.C., 30 December 1977.
- 15. U.S. Army, Office of the Chief of Engineers, Engineer Technical Letter No. ETL 1110-2-234, "Engineering and Design, National Program of Inspection of Non-Federal Dams, Review of Spillway Adequacy," Corps of Engineers, Washington, D.C., 10 May 1978.
- 16. U.S. Department of Commerce, "Technical Paper No. 40, Rainfall Frequency Atlas of the United States for Durations from 30 Minutes to 24 Hours and Return Periods from 1 to 100 Years," Weather Bureau, Washington, D.C., May 1961.
- 17. U.S. Department of Commerce, National Oceanic and Atmospheric Administration, "Hydrometeorological Report No. 51, Probable Maximum Precipitation Estimates, United States East of the 105th Meridian," Washington, D.C., June 1978.

NAME OF DAM: LAKE HOLLY DAM

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